



Rigid Pavement Repair

Craig Hennings

American Concrete Pavement Association -
Southwest





IF YOU'D CARRIED AS MANY LOADS AS IT HAS YOU'D BE SHOWING YOUR AGE, TOO.



DEVELO 98

PCCP Repair Techniques

- Full-depth repair
- Partial-depth repair
- Slab stabilization
- Retrofitting dowels
- Cross-stitching longitudinal cracks/joints
- Diamond grinding
- Joint & crack resealing

Concrete Pavement Preservation

- First level of response for deteriorating concrete pavements should always be PCC Preservation
 - Least cost – Cheaper than reconstruction
 - Least service disruption
 - Increases safety
 - Environmentally sound
 - Addresses operator comfort

Factors to Consider

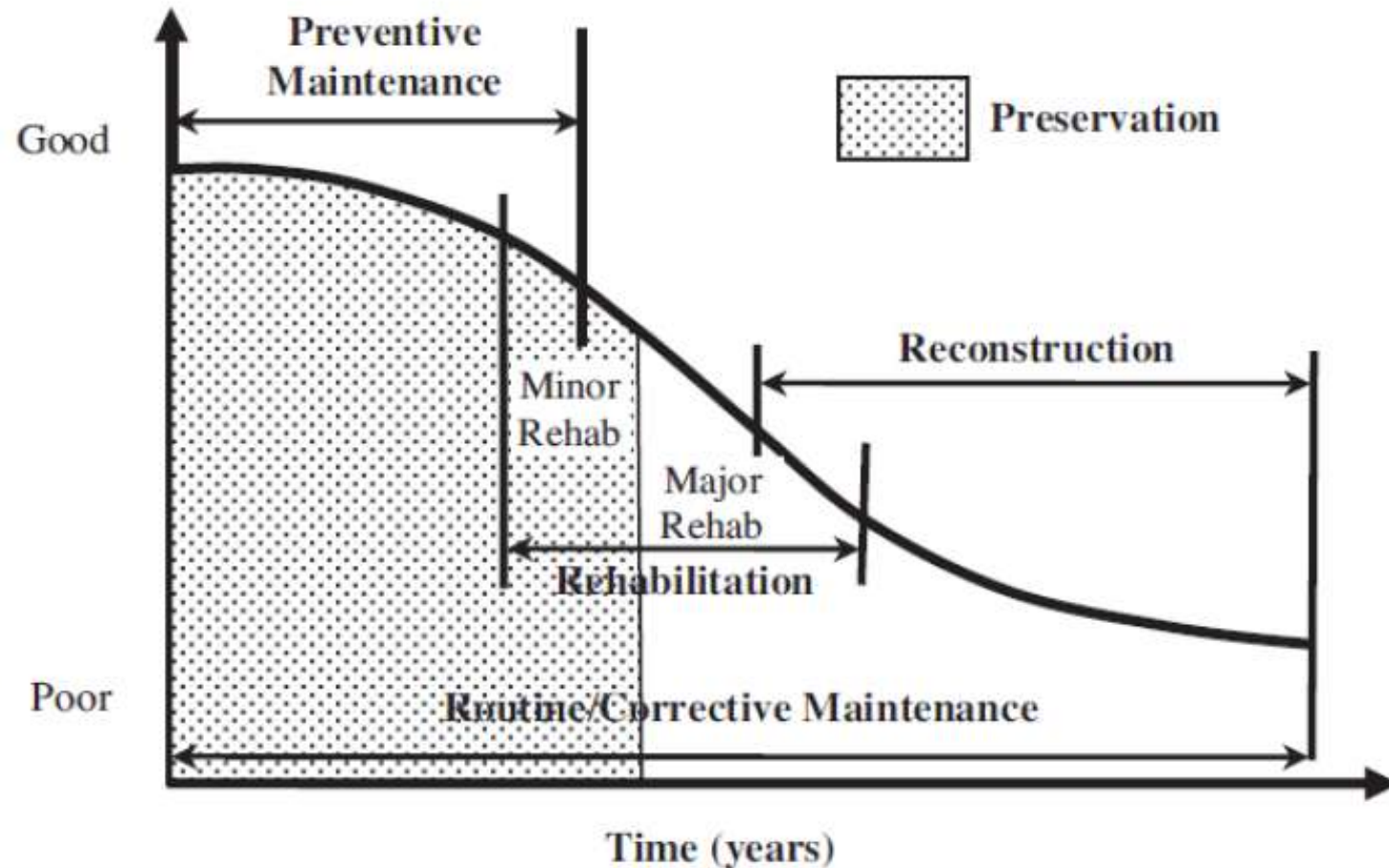
- Distress Types
- Ride
- Skid
- Noise
- Durability/Longevity

Purpose of Concrete Pavement Repair

- Used early when pavement has little deterioration.
 - Repairs isolated areas of distress.
 - Repairs some construction defects.
 - Manages the rate of deterioration.



Traditional Pavement Management



Traditional Concrete Pavement Preservation

Treatment	Expected Performance	
	Treatment Life (yr)	Pavement Life Extension (yr)
Concrete joint resealing	2-8	5-6
Concrete crack sealing	4-7	NA
Diamond grinding	8-15	14 - 17
Diamond grooving	10-15	NA
Partial-depth concrete patching	5-15	NA
Full-depth concrete patching	5-15	NA
Dowel bar retrofitting	10-15	NA
Ultra-thin bonded wearing course	6-10	NA
Thin HMA overlay	6-10	NA

Sources: Peshkin et al. 1999; Smith et al. 2008; Peshkin et al. 2007; Galtrans 2008; NDOR 2002.

Note: NA = Not available.

Is Sealant Cost Effective?

FHWA Sealant Effectiveness Study

TechBrief

The Concrete Federal Highway Administration (FHWA) is currently conducting a study to evaluate the long-term performance of concrete pavement, managed by the Federal Highway Administration through a program with state highway agencies, industry and academia, to improve performance and reduce costs. The program has identified to provide state-of-the-art information on concrete pavement and other technologies in order to assist state highway agencies in making decisions on how to improve and maintain the quality of concrete pavements.

www.fhwa.gov/concrete/techbrief/



U.S. Department of Transportation
Federal Highway Administration



Performance of Sealed and Unsealed Concrete Pavement Joints

This TechBrief presents the results of a nationwide study of the effect of transverse joint sealing on performance of jointed plain concrete pavement (JPCP). This study was conducted to assess whether JPCP designs with unsealed transverse joints performed differently from JPCP designs with sealed transverse joints. Data on crack and deflection data were collected from 177 test sections of 28 experimental joint sealing projects in one to 11 states. Performance of the pavement test sections with unsealed joints was compared with the performance of pavement test sections with unsealed types of sealed joints.

BACKGROUND

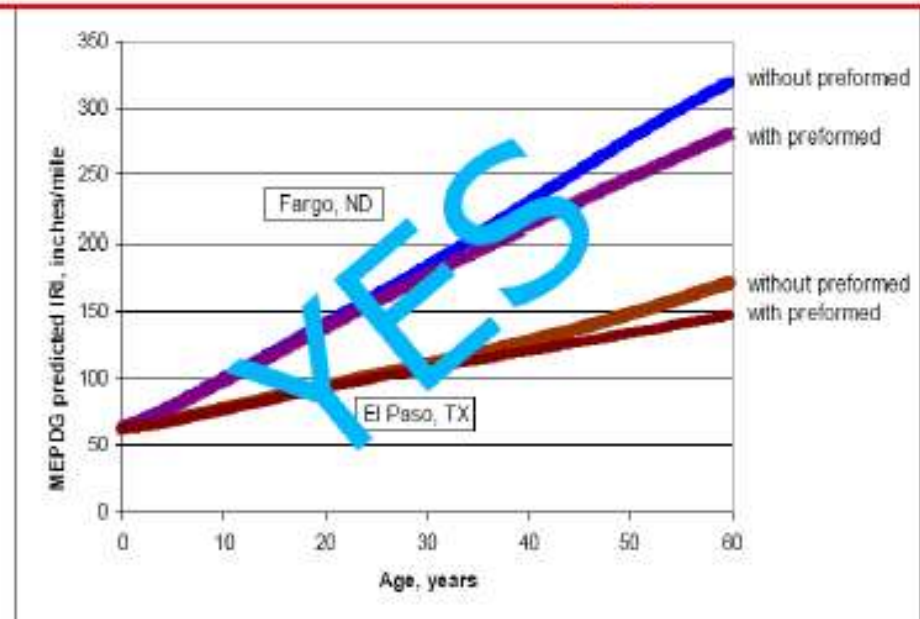
The sealing of transverse contraction joints in JPCP has been standard practice throughout much of the United States for many years. An widespread reason is due to the concern that dust coating joints may increase moisture pavement performance in two ways: by reducing water infiltration into the pavement structure, thus reducing the occurrence of moisture-related distresses such as pumping and cracking, and by preventing the infiltration of incompressible dirt, sand and gravel into the joints, thereby reducing the likelihood of aggregate-related distresses such as joint spalling and blowups.

Transverse contraction joints in concrete pavement (JCP) are typically created by making an initial saw cut, followed by controlled cracking, followed by a second, wider saw cut to produce a reservoir for the joint sealant material. This traditional approach of sawing and sealing transverse contraction joints is estimated to account for between 2 and 7 percent of the initial construction cost of a JCP. Moreover, these sealed transverse joints require resealing one or more times over the service life of the pavement, leading to additional costs in terms of labor, materials, operations, and lane closures.

Recently, several state departments of transportation (DOTs) have been questioning conventional transverse joint sealing and sealing practices. These agencies concluded that the benefit derived from sealing does not offset the costs associated with the placement and subsequent curing of the sealant over the life of the pavement. As a result, they have been experimenting with different sawing and sealing alternatives, for example:

- narrow unsealed joints, consisting of single saw cuts that are left unsealed;
- narrow filled joints, consisting of single saw cuts that are filled with sealant that adheres to the sides and bottom of the saw cut;
- narrow sealed joints, consisting of single saw cuts that contain a narrow backing rod and sealant material.

AASHTO New Design Guide

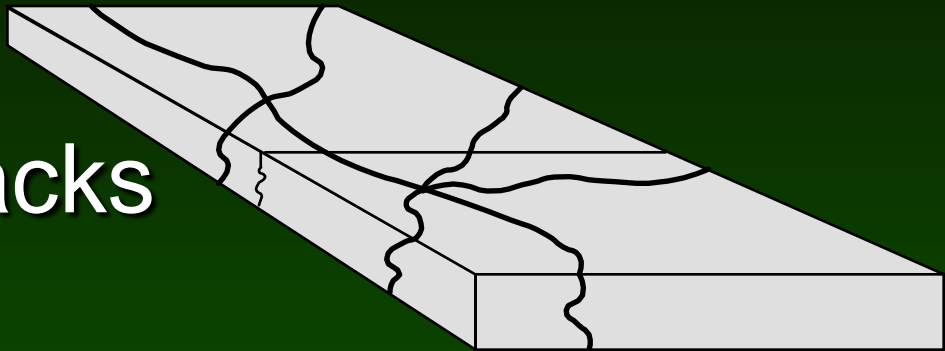


PCCP Repair Techniques

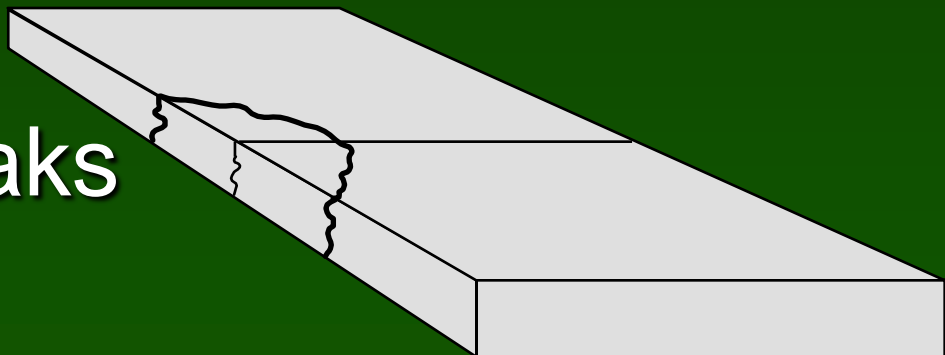
- Full-depth repair
- Partial-depth repair
- Slab stabilization
- Retrofitting dowels
- Cross-stitching longitudinal cracks/joints
- Diamond grinding
- Joint & crack resealing

Full-Depth Repair

Multiple Cracks



Corner Breaks





Full Depth
Patching



RAPID STRENGTH CONCRETE FOR PAVEMENTS

•Rapid strength concrete (RSC) has been efficiently used for emergency repair and planned rehabilitation of concrete infrastructure and for new construction, where acceleration is a concern. This concrete is produced with hydraulic cements. Accelerators of hardening, if used, are non-chloride in nature.



•Emergency rehabilitation of truck bypass tunnel,
•I-5 and RTE-14, November 2007.

•Total volume of various types of rapid strength concrete specified for construction in 2008 by Caltrans exceeded 190,000 cubic yards. Volume of rapid strength concrete specified for pavement rehabilitation was almost 130,000 cubic yards. Volume of rapid strength lean concrete base was approximately 37,000 cubic yards.



•Pavement replacement at I-10, Pomona, CA, 2009 (4-hrs RSC)



•Replacement HOV lane at I-10, Los Angeles, CA, 2009 (12-hrs RSC)



•Pavement replacement at RTE-710, Los Angeles, CA, 2005 (1.5-hrs RSC)



•The principle difference in required performance of regular and rapid strength concrete is the time factor. A ten minute delay in opening temporarily closed freeway lanes to traffic may delineate a successful pavement repair shift from one for which the contractor would be penalized.

Proportioning for ultra-rapid strength gain in early age is one difference in the design of RSC and regular concrete.

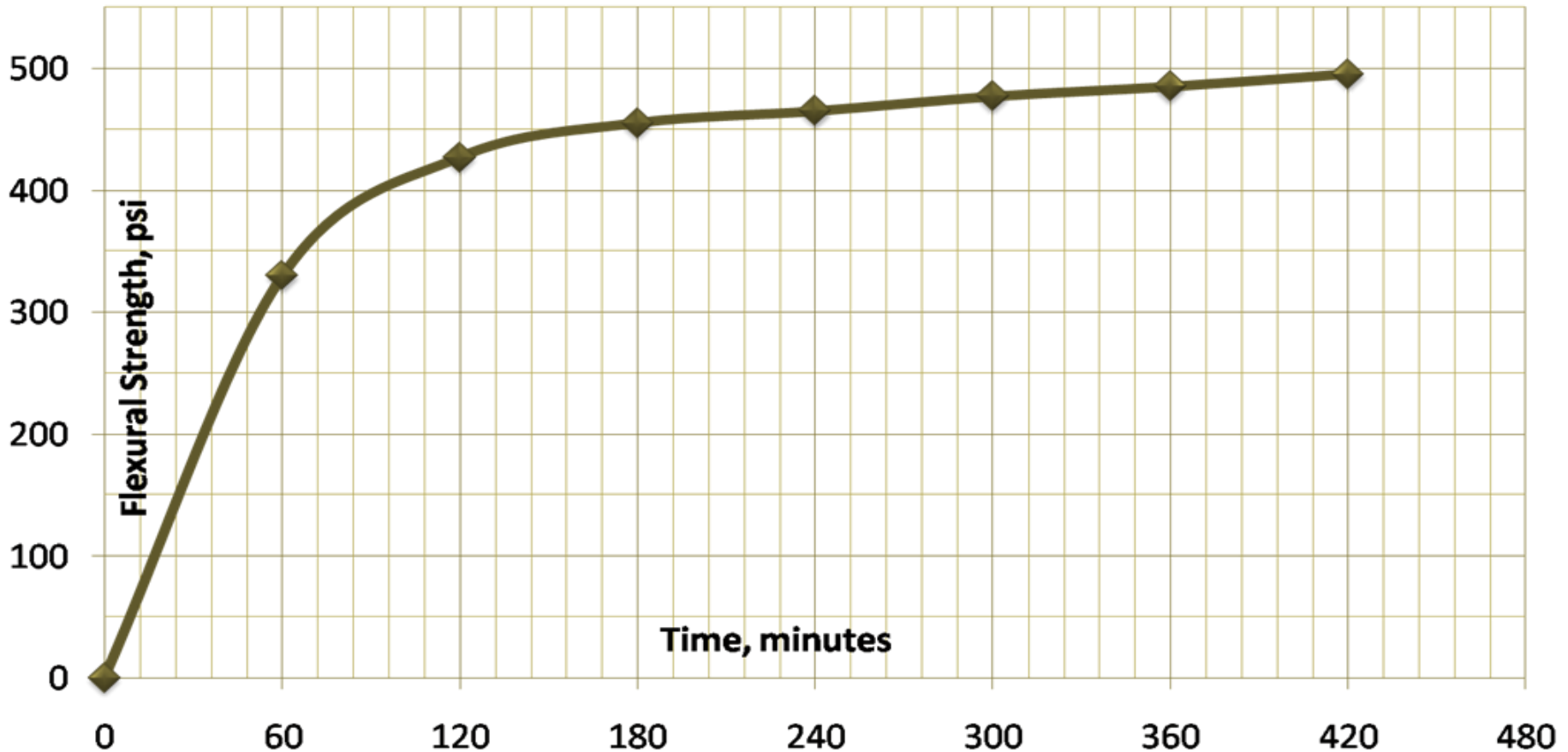
Options:

- **Use of faster hardening hydraulic cements (rapid hardening cements, ASTM C1600; Type III portland cement, ASTM C150),**
- **Use of accelerating admixtures (non-chloride),**
- **Limiting W/C,**
- **Optimizing (increasing) initial and curing temperatures of concrete.**

Proper design of RSC should account for other properties influencing acceleration of construction, such as:

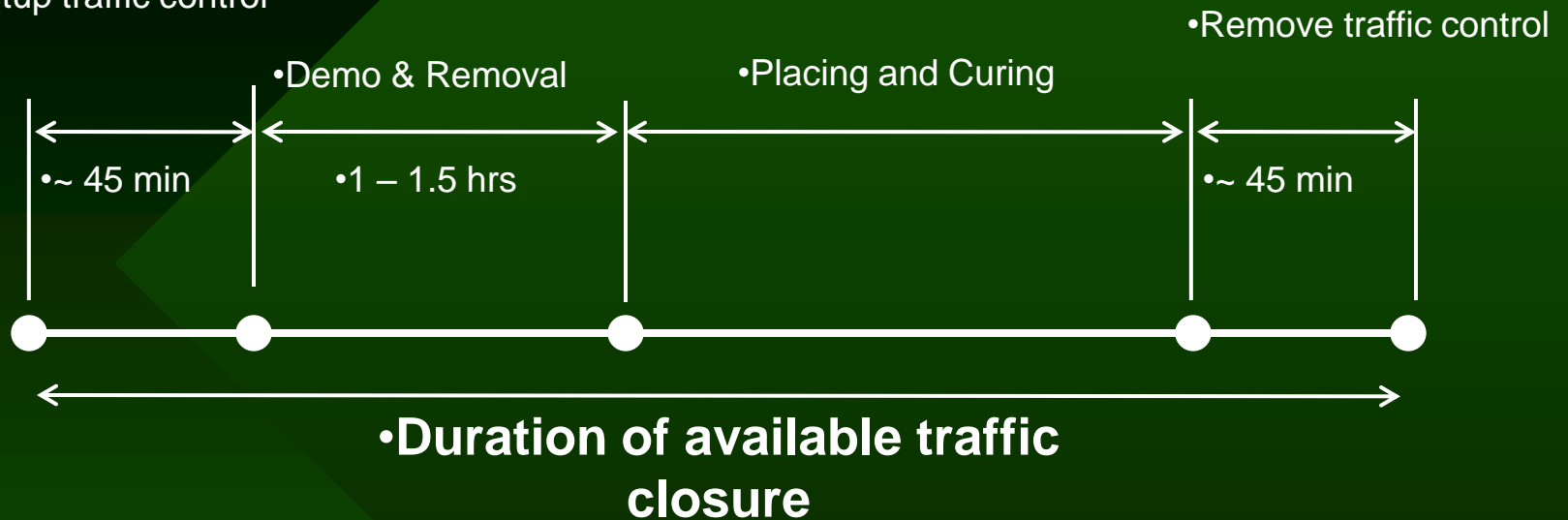
- **Ability to be placed, spread and consolidated conveniently and quickly without segregation,**
- **Time within which fresh RSC retains workable consistency,**
- **Ability to be finished promptly upon completion of consolidation,**
- **And more.**

- Required rate of hardening of RSC depends on the specified strength and time allowed for its achieving.



•Required rate of strength gain of RSC for pavement replacement depends on the maximum allowed curing time which is largely determined by available closure time and by desired productivity (volume of RSC placed per shift).

- Setup traffic control



Duration of Traffic Closure	Duration of Placing and Curing
6 hours	~ 3.5 hours
8 hours	~ 5.5 hours
10 hours	~ 7.5 hours
12 hours	~ 9.5 hours

RSC Design Notes

- **Two types of rapid strength concrete (RSC) mostly used in California for pavement rehabilitation within short-time lane closures are:**

- **RSC with rapid hardening cements (examples of such cements are CTS Rapid Set[®] Cement and Ultimax Cement-DOT[®])**
- **RSC with Type III Portland cement and non-chloride accelerator of hardening (this type of RSC is often called “4 x 4” concrete, because it was first developed to achieve flexural strength of 400 psi in 4 hours)**

•The referenced RSC types allow for achieving flexural strength of 400 psi required for opening lanes to traffic (and corresponding compressive strength of \geq 2,500 psi) as quickly as:

- 1-hour plus when rapid hardening cements are used**
- 2.5-hour plus when Type III portland cement with accelerators of hardening are used**

RSC for pavements is designed for:

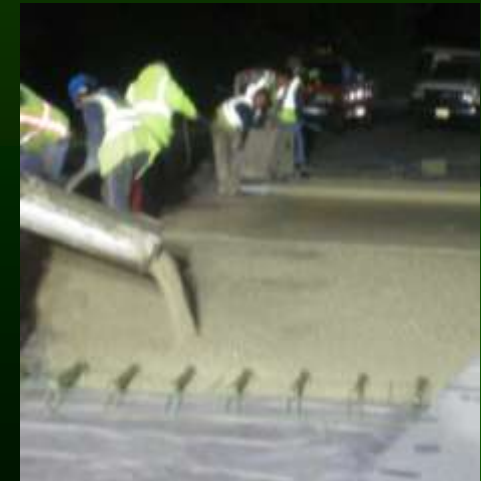
- **Workability**
 - **Consistency**
 - **Cohesiveness**
 - **Time within which fresh RSC retains workable consistency**
- **Ability to be finished promptly after placement**
- **PROPORTIONING OF RSC**

RSC for pavements is designed for (continued):

- **Flexural strength**
 - **Maximum curing time prior to opening lanes to traffic**
 - **Minimum flexural strength prior to opening to traffic**
- **Temperature of application**
- **Freezing and thawing resistivity**
- **Shrinkage**
- **Resistance to sulfate exposure**

- **PROPORTIONING OF RSC**

•RSC is typically proportioned with superplasticizers for achieving desired (often near-flowable) consistency while maintaining low water to cement ratio (W/C). Hydration controlling admixtures extend time within which RSC retains workable consistency. Optimized consistency and cohesiveness accelerate construction of pavements.



•WORKABILITY

- Limited bleeding of RSC allows for prompt finishing. Fast setting of RSC and subsequent accelerated gain of tensile strength mitigate risk of plastic shrinkage cracking associated with the use of low-bleeding concrete.



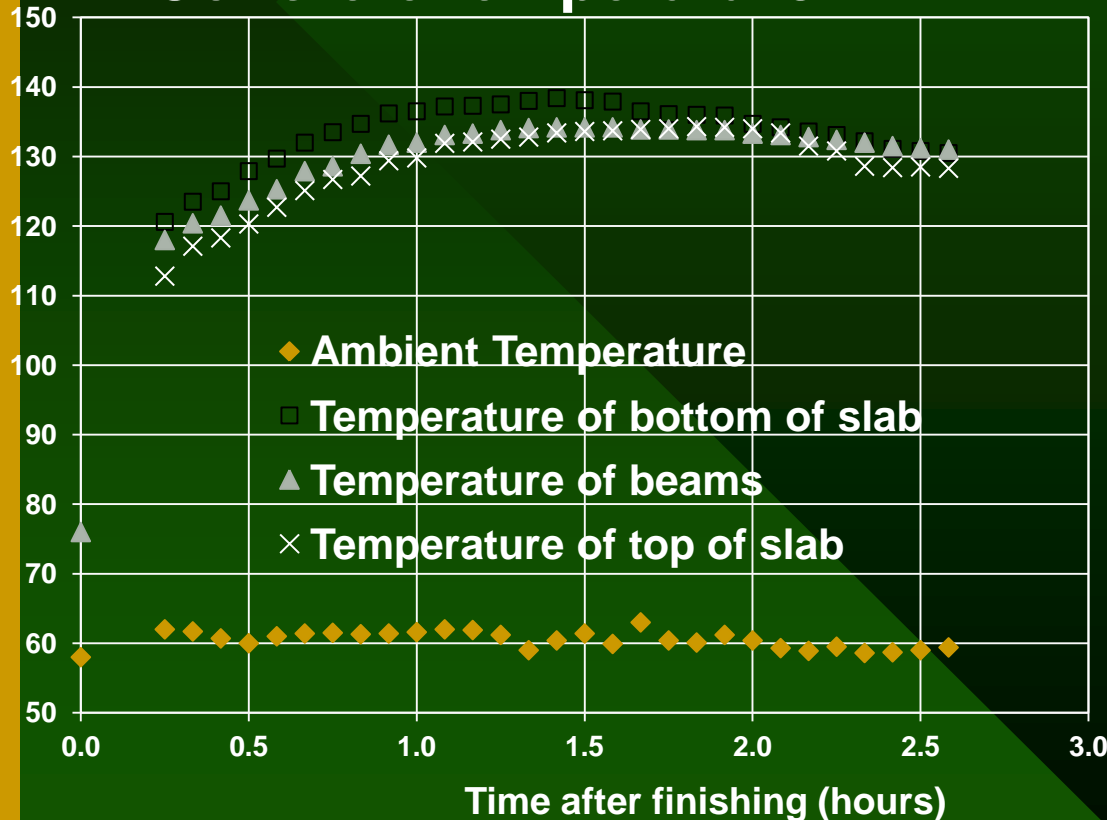
• FINISHABILITY

•Rate of strength gain of RSC is mainly controlled by:

- ✓ Type of hydraulic cement
- ✓ Chemical admixtures
- ✓ Water to cement ratio
- ✓ Concrete temperature

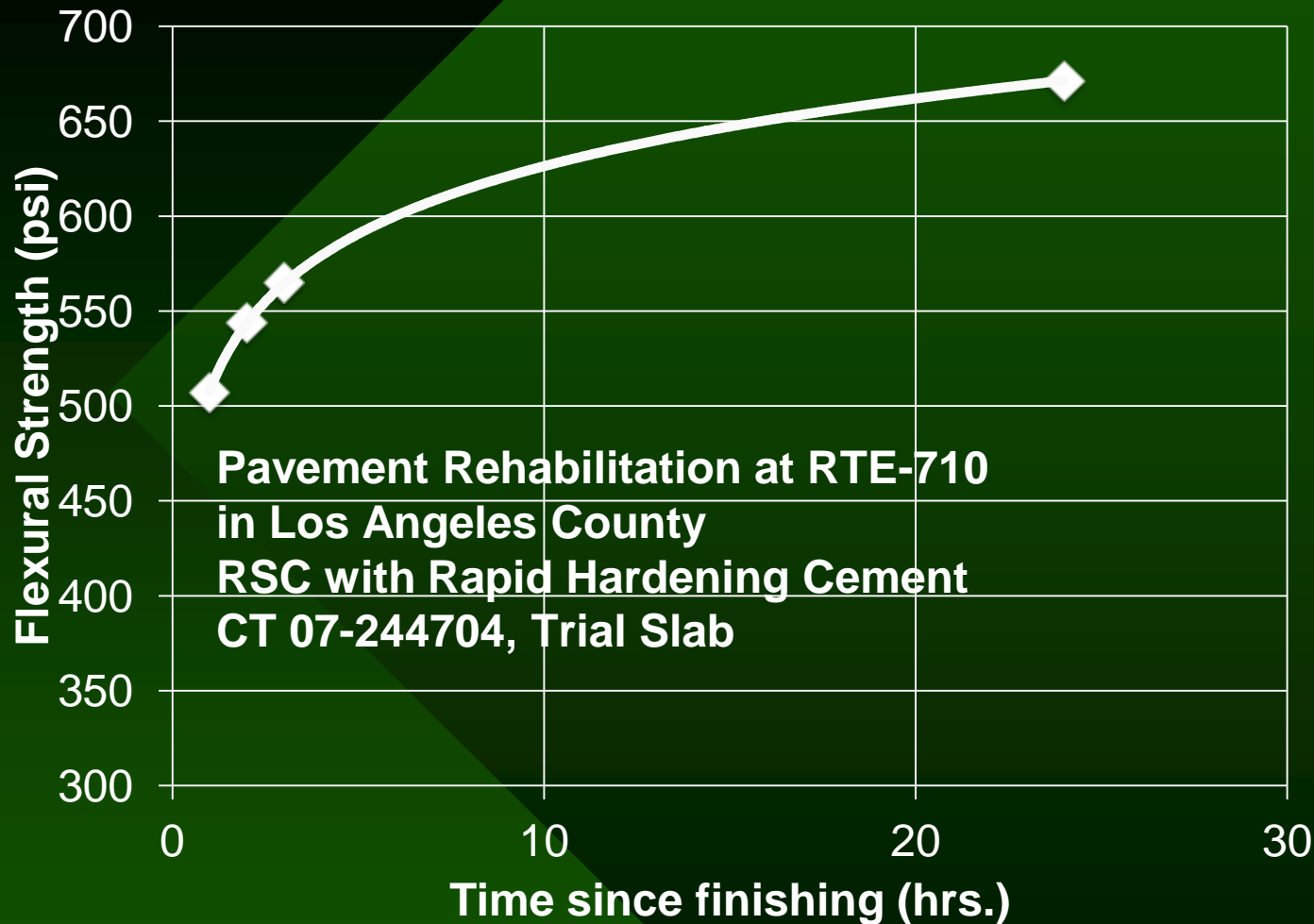
• Testing Notes:

• Estimation of strength gain requires matching temperature of concrete in specimens and in pavement during curing.



Min Curing Time to Achieve MOR (*)		Proportioning for Early Age Flexural Strength		
400 psi	550 psi	Type of Cement	Accelerator , fl. oz./100 # cmt.	Max W/C
1 to 2 hours	2 to 4 hours	Rapid hardening cement, ASTM C1600	-----	~0.41 - 0.43
2.5 to 4 hours	4 to 7 hours	Portland cement Type III, ASTM C150	70 to 100	~0.32 - 0.34
8 to 12 hours	12 to 16 hours	Portland cement Type III, ASTM C150	20 to 40	~0.34 - 0.36
≥16 hours	≥22 hours	Portland cement Type II, ASTM C150	None	~0.37 - 0.39

•NOTE: (*) Since the time RSC has been formed and finished.



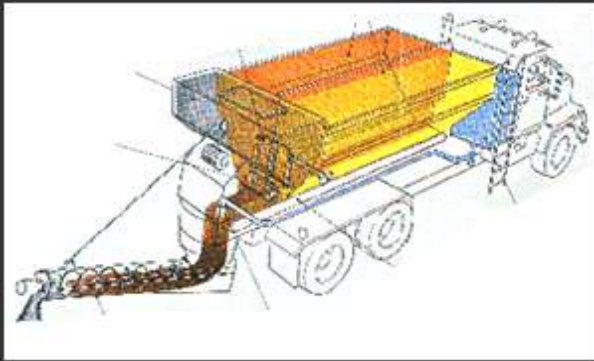
•Rapid strength concrete mix design should be validated by construction and testing of a trial slab. Ambient conditions during construction of the trial slab shall be representative of those anticipated during construction.

RATE OF STRENGTH GAIN IN EARLY AGE



Production Notes

- RSC with rapid hardening cements is most often produced by mobile (volumetric) mixers to
- allow for immediate placement. The demand in hydration stabilizers is reduced and uniformity of workability and strength is improved.



- RSC with Type III portland cement typically has been produced using transit mixers. Superplasticizer and set controlling admixture are added at the batch plant. Accelerator of hardening is added on site.



PRODUCTION NOTES

Paving Notes

Time within which RSC retains workable consistency is limited. It may be as short as 15 minutes (typical for RSC with rapid hardening cements designed to achieve flexural strength of 400 psi in 1-hour).

Paving crew should promptly place, consolidate, screed off, finish and texture concrete.

•PAVING NOTES

Reduced bleeding eliminates “waiting time” prior to finishing. Rapid development of strength reduces risk of plastic shrinkage cracking. It is still important to protect finished and textured surface promptly with curing compound to provide for more complete hydration and to assure abrasion resistance

•PAVING NOTES



•Inadequate curing and protection, as well as excessive application of water to the surface, may reduce abrasion resistance of RSC pavement.



•PAVING NOTES

Concrete temperature is critical for workability retention. In hot conditions, consistency (penetration) will decrease faster within time.

The time within which fresh concrete retains workable consistency can be controlled by using hydration stabilizers. Use of hydration stabilizers should be optimized with respect to both workability retention and early age strength gain of RSC.

•PAVING NOTES



Temperature influences strength gain of RSC. Higher the temperature, faster the strength gain; lower the temperature, slower the strength gain.

RSC shall be designed for specific temperature of application, which is one of the most important constructability considerations.

•PAVING NOTES



•Concrete supplier shall recommend the minimum initial temperature of RSC, especially when work is performed during cooler weather periods. We recommend the RSC temperature shall not be less than 65°F prior to placement (preferably not less than 70°F). Use of hot water is one efficient option used by some RSC suppliers.

•PAVING NOTES



RSC sets and hardens dramatically faster than regular portland cement concrete. Sawing window starts earlier and is shorter than for regular pavement concrete.

•Special Provisions by Caltrans provide for saw cutting no later than in 2 hours.

•PAVING NOTES



Concrete temperature is critical for strength gain. Mass of RSC in a pavement slab is significantly larger than in a test beam. In early age, the temperature of concrete in pavement slab is normally higher than in an unprotected test beam.

•Beams shall be cured in heat retaining enclosures for matching the pavement slab temperature.



•TESTING NOTES



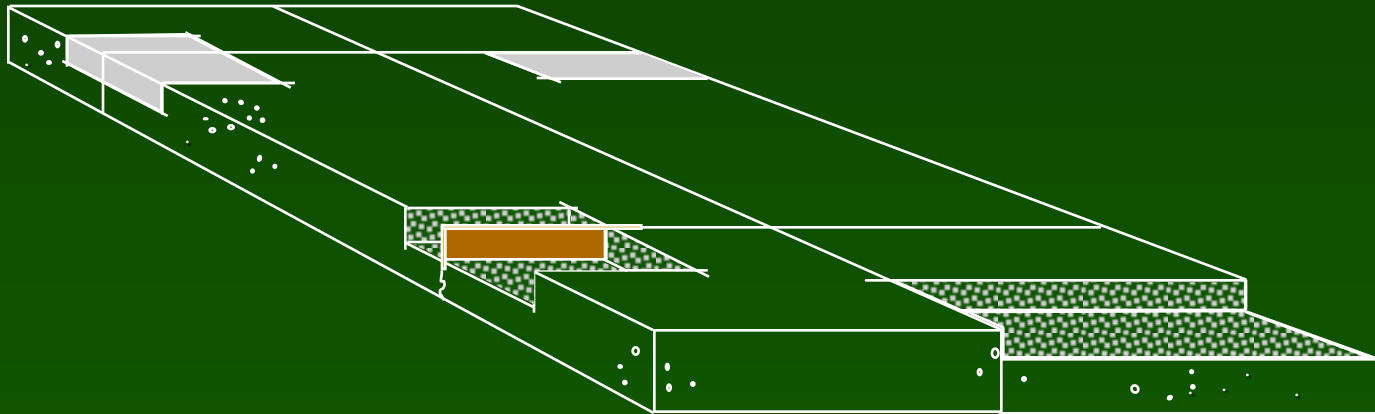
Removal for Full Depth Repairs



- Non-impact method
 - Backhoe or lifting devices
 - Preserves existing base
 - Prevents damage to adjacent slabs

Partial Depth Repairs

- Repairs deterioration in the top 1/3 of the slab.
- Generally located at joints, but can be placed anywhere surface defects occur.





Partial Depth Patching



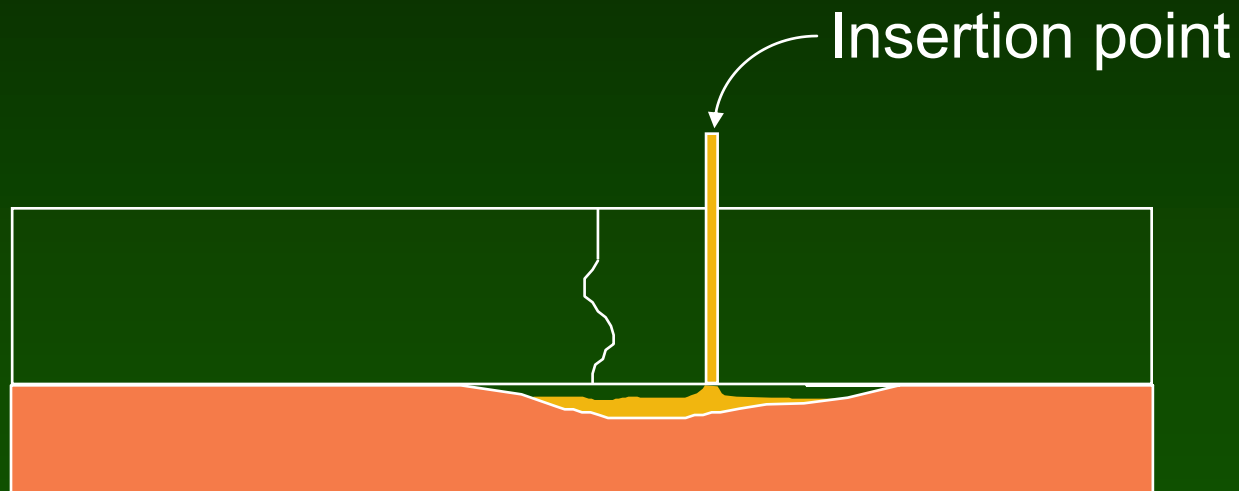


I-10 Indio, CA Aug 2010

courtesy of CeraTech

Slab Stabilization / Slab Jacking

Pressure insertion of flowable material beneath the PCC slab





Slab
Stabilization/
Jacking

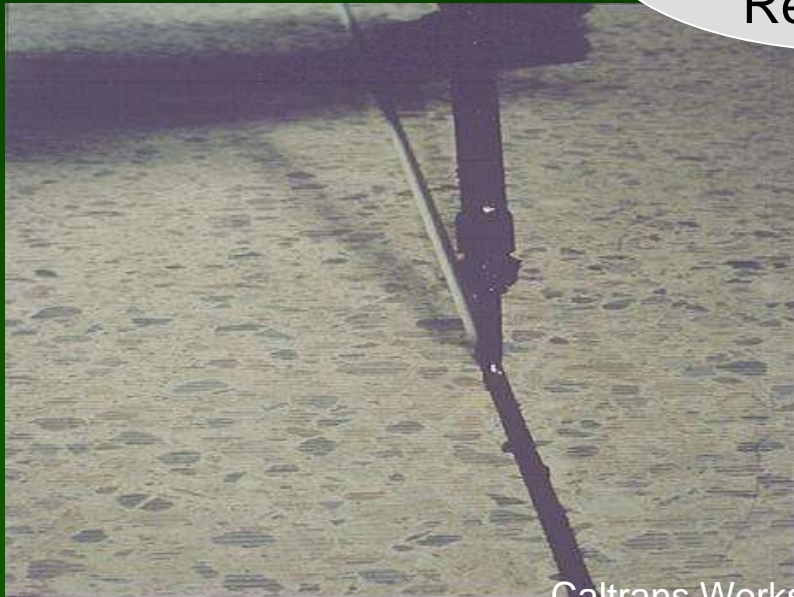


Joint/Crack Resealing

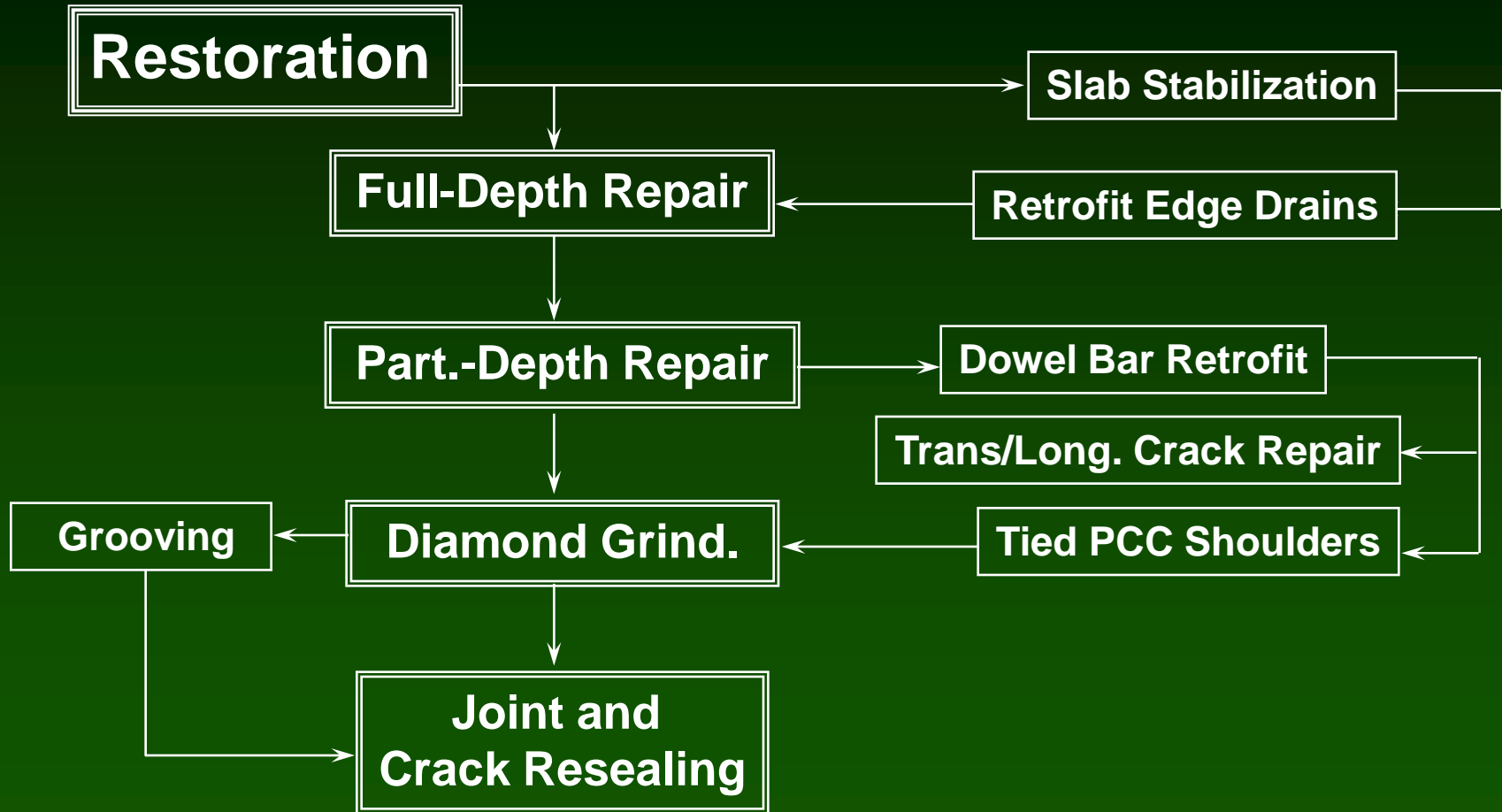
- Application of a sealant material in concrete pavement joints and cracks
- Purpose
 - Minimize moisture infiltration
 - Prevent intrusion of incompressibles
- Sealant Materials
 - Rubberized asphalt
 - Silicone



Joint and
Crack
Resealing



Sequencing of Techniques



Repair/Rehabilitation Strategy Selection

Determining correct strategy is NOT complicated.

- Determine the cause of distress.
 - Structural, Functional , Material, Drainage
- Consider multiple perspectives
 - Ride Quality, Traffic, Noise, Maintenance Requirements, Lane-Condition Uniformity, Future Performance, Cost

DOT Approach to Selecting Maintenance Treatments

- Assess Existing Pavement Conditions
 - Conduct visual site inspection and/or review project information
 - Perform testing on the existing pavement, as conditions require
 - Define the performance requirements for the treatment

Pavement Distress Types

Concrete Pavement

- Structural Distress
 - Cracking
 - Joint / Crack deterioration
 - Durability distress
 - Punchouts (CRCP)
- Functional Distress
 - Faulting
 - Noise
 - Surface polishing & surface defects

DOT Approach to Selecting Maintenance Treatments

Data Item	Grind-ing	Load Transfer Restor-ation	Partial-Depth Repair	Full-Depth Repair
Existing Pavement Structure	X	X	X	X
Original Construction Data	*	*	*	*
Age	*		*	*
Materials Properties	X		*	*
Subgrade				
Climate				
Distress	X	X	X	X
Skid	*			
Accidents	*			
NDT		X		*
Destructive Testing/ Sampling	*	*	X	X
Roughness	*			
Surface Profile	X			
Drainage	X			X
Previous Maintenance	*		*	*
Utilities				X
Traffic Control Options	X	X	X	X

Proposed Trigger Values and Expected Life

Treatment	Trigger (National)	Climate Region ¹				Traffic ADT			Life of Treatment (Year)	Estimated Cost (\$)²
		Desert	Valley	Coastal	Mountain	<5000	>5000, <30000	>30000		
Crack Resealing	>1/4 inch	>1/4	>1/4	>1/4	>1/4	>1/4	>1/4	>1/4	4 - 7	\$27.7k - 42.4 k/ln mi
Diamond Grinding	Faulting > 1/4 inch; Ride 95 in/mile	>1/4 >190	>1/4 >95	>1/4 >95	>1/4 >190	>1/4 >190	>1/4 >125	>1/4 >95	10 - 18	\$30.0k - 80.1k/ln mi
Partial Slab Repair	Surface distress - Patches <1.2 yd²	<1.2	<1.2	<1.2	<2.4	<2.4	<1.2	<1.2	8 - 12	\$135 - 270/yd³
Isolated Slab Replacement	3rd stage cracking or unstable slabs	Same Trigger Value. For desert, mountain, or ADT<5000, District makes decision to repair.							8 - 12	\$4000 - \$8000/slab
Dowel Bar Retrofit	LTE <60%, Faulting>1/4 inch, Max 10% Cracking	<40 >1/4 20	<70 >1/4 10	<70 >1/4 10	<50 >1/4 20	<50 >1/4 20	<70 >1/4 10	<70 >1/4 10	8 - 17	\$141k - 177k/ln mi

Approach for Selecting Maintenance Treatments

Decision Factor Names ⇨	DECISION FACTORS						Total Score	Rank
	Initial Cost	Life Cycle Costs	Expected Life	Ease of Repairing/ Maintaining	Construction Traffic Control	Proven Design in Agency		
Weightings ⇨	25	15	20	15	10	15		
Alternative 1	60 15	60 9	100 20	80 12	90 9	100 15	80	1
Alternative 2	60 15	60 9	100 20	80 12	90 9	100 15	80	1
Alternative 3	60 15	60 9	70 14	50 7.5	60 6	40 6	57.5	6
Alternative 4	60 15	60 9	70 14	50 7.5	60 6	40 6	57.5	6
Alternative 5	60 15	40 6	100 20	80 12	100 10	90 13.5	76.5	3
Alternative 6	60 15	80 12	40 8	20 3	40 4	20 3	45	9
Alternative 7	40 10	60 9	40 8	50 7.5	50 5	30 4.5	44	10
Alternative 8	70 17.5	80 12	60 12	50 7.5	80 8	40 6	63	5
Alternative 9	100 25	100 15	20 4	20 3	40 4	40 6	57	8
Alternative 10	30 7.5	60 9	100 20	100 15	100 10	30 4.5	66	4

Summary

- Many available treatments for PCC pavements
- Each has advantages and limitations
- Performance and cost vary with given conditions
- Applying the right treatment to the right pavement *at the right time*
- No universal method available
- Take advantage of local contractor experience
- ACPA is ready to assist